

#### **AECOM**

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#### **Technical Memorandum**

**Date:** June 11<sup>th</sup>, 2010

To: Brian Beyak, P.Eng, Manitoba Hydro

From: Jamie Ellis, P.Eng, AECOM

Project Number: 0217-200-07 (3) [Hydro Ref: 243 981 0100]

Subject: Keeyask Infrastructure Project - North Access Road Start Up Camp

Design for Wastewater Disposal

Additional Information at June 11<sup>th</sup>, 2010

Distribution: Neil Klassen, C.E.T., AECOM

Bob Romanetz, P.Eng, AECOM

#### 1. Introduction

Information relating to wastewater disposal from the North Access Road Start Up Camp (SUC) was previously submitted as part of the Environment Act Proposal submission for the Keeyask Infrastructure Project. Concept design information was presented in the following technical memoranda submissions:

- Original (dated July 28th, 2009)
- Revision 1 (dated October 2<sup>nd</sup>, 2009)
- Revision 2 (dated October 21<sup>st</sup>, 2009)
- Additional Information (dated October 26<sup>th</sup>, 2009)

Following a telephone discussion between Rob Boswick (MB Conservation) and Jamie Ellis (AECOM) on April 28<sup>th</sup>, 2010 this memo has been prepared to show changes made during the development of detailed design, as additional information has become available.

#### 2. Accompanying Drawings

The following drawings relating to the septic tanks and drain field are attached;

1001 – General Plan
 1004 – Drain Field - Detailed Site Plan
 1007 – Plan/Profile – Forcemain and Drain Field
 1009 - Drain Field Grading Plan
 2006 – Sewer and Water Details 5
 1010 – Sewer and Water Details 6

#### 3. Proposed Changes from Concept Design

#### a) Type of Field

During the concept design stage our calculations indicated that a trench type disposal field utilizing effluent chambers would create the smallest total footprint for the drain field. These calculations were based on the Manitoba Environment Act Regulations at the time. Since that time amendments to the Regulations have been issued (amendment 156/2009). These amendments affect the calculations used to determine the size of trench type disposal fields utilizing chambers by removing the "Open Area Multiplier" from the calculation for length of trench required. It is understood that this multiplier was in place to acknowledge the larger storage capacity of a chamber vs. traditional aggregate system.

Therefore, sizing calculations were re-examined and it was found that a total area field utilizing effluent chambers now created the smallest foot print, rather than a trench field. This combined with a small change to the location of the field has allowed for a reduction in earthworks quantities.

It should be noted however, that more chambers will be required. It is proposed that the chambers are laid level in the east to west direction. From discussions with Rob Boswick, it is also understood that only the two types of chamber approved for use in Manitoba would be considered acceptable (Infiltrator and ADS brands).

#### b) Application rate

The majority of the soil samples tested for gradation correspond to a silt loam to a loamy sand given the low percentage of clay (i.e. less than 10 percent) and the range of silt and sand percentages. As can be seen from the table in Enclosure 2, whilst some samples did contain greater than 85% sand content, these are at least 1m below the base of the field, and TH09-13 is some 75m away from the west end of the field. Results are summarized on the Soil Texture Classification Triangle contained within the Supplementary Information as shown in **Figure 1** (attached). As shown, application rates range from 12.72 to 29.35 litres/m²/day. Therefore, the wastewater application rate assumed in the concept design (12.72 litres/m²/day) was revised to 18 litres/m²/day. Whilst this is not a value within the Soil Texture Classification Triangle, it is considered to be more appropriate and representative of the existing soil conditions. Again, these changes allowed for a reduction in earthworks required for the drain field.

Based on the above, the following equation was used to size the field which gives a field area requirement of 5,000 m<sup>2</sup>.

$$Area of Field = \frac{(Daily Effluent Flow) \times (Safety Factor)}{Application Rate}$$

This area has been provided over 4 cells, each with 4 zones measuring 8.1m by 39.0m, giving a total field area of approximately 5060 m<sup>2</sup>.

#### c) Bed level

As described in previous information, three test holes were completed in the field location.

The upper 1.5 m of soil at Test Holes 09-13, 09-14 and 09-15 generally consists of a silt with trace to some sand and variable clay content (trace clay to clayey). Below this layer the soil has variable silt and sand content (i.e. silt content ranges from about 11 to 85 percent and the sand ranges from about 7 to 88 percent) and the clay content is less than 10 percent.

The review of the information suggested that the base of the field could be located higher than previously assumed in the concept design drawings (the potential for this change was noted in Revision 2 of the Technical Memorandum).

#### 4. Pressure system

From discussions with Manitoba Conservation, it was understood that a gravity system would not be as considered appropriate for a field of this size, as even distribution of wastewater across the whole bed area could not be guaranteed. Therefore, a calculation method provided within guidance documents from Alberta (Private Sewage Systems Standard of Practice Handbook, 1999) has been used to size the pressure distribution system, comprising headers and laterals.

An at-grade package lift station is proposed rather than submersible pumps. A pressure filter will also be installed to remove small debris (such as hair) that could clog small holes on the distribution laterals. These filters will require regular cleaning from camp maintenance staff.

As shown on the drawings, it is proposed that an arrangement of 5 automatic distributing valve assemblies (1 primary and 4 secondary, each mechanically operated) which operate with each pump cycle. As such, flow is sequentially directed to a total of 16 zones across the 4 cells.

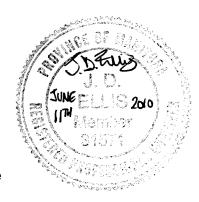
In addition, due to the shallow nature of the system, a number of freeze protection measures are included, including box insulation and placement of stripping material (organics) over the chambers.

#### 5. Conclusion

In summary, this memo provides details of improvements which have occurred between concept and detailed design. These include the use of a total area field, more chambers, a higher application rate and a pressure distribution system.

**Technical Memorandum** June 11<sup>th</sup>, 2010 Page 4

Respectfully submitted,



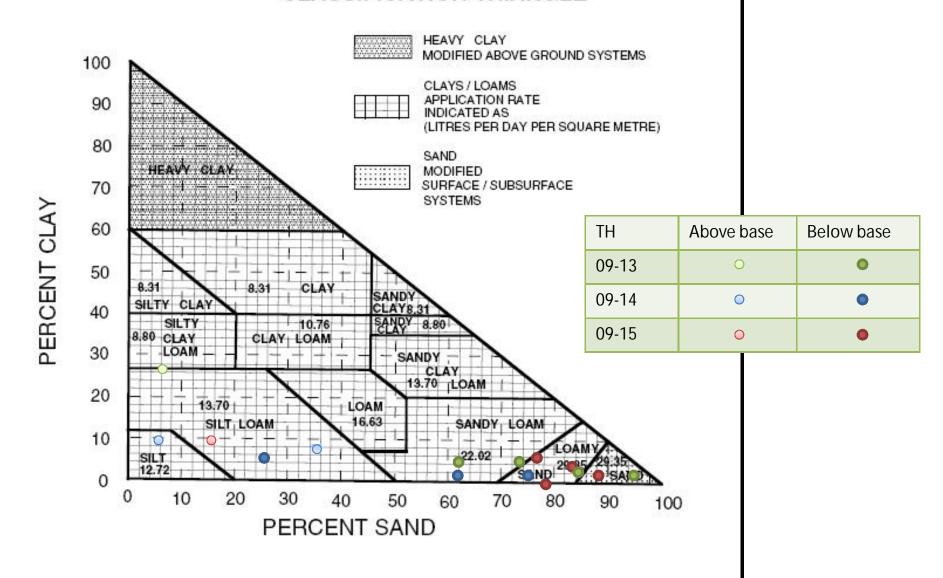
J. D. Ellis, P.Eng Community Infrastructure AECOM Canada Ltd.

Encs:

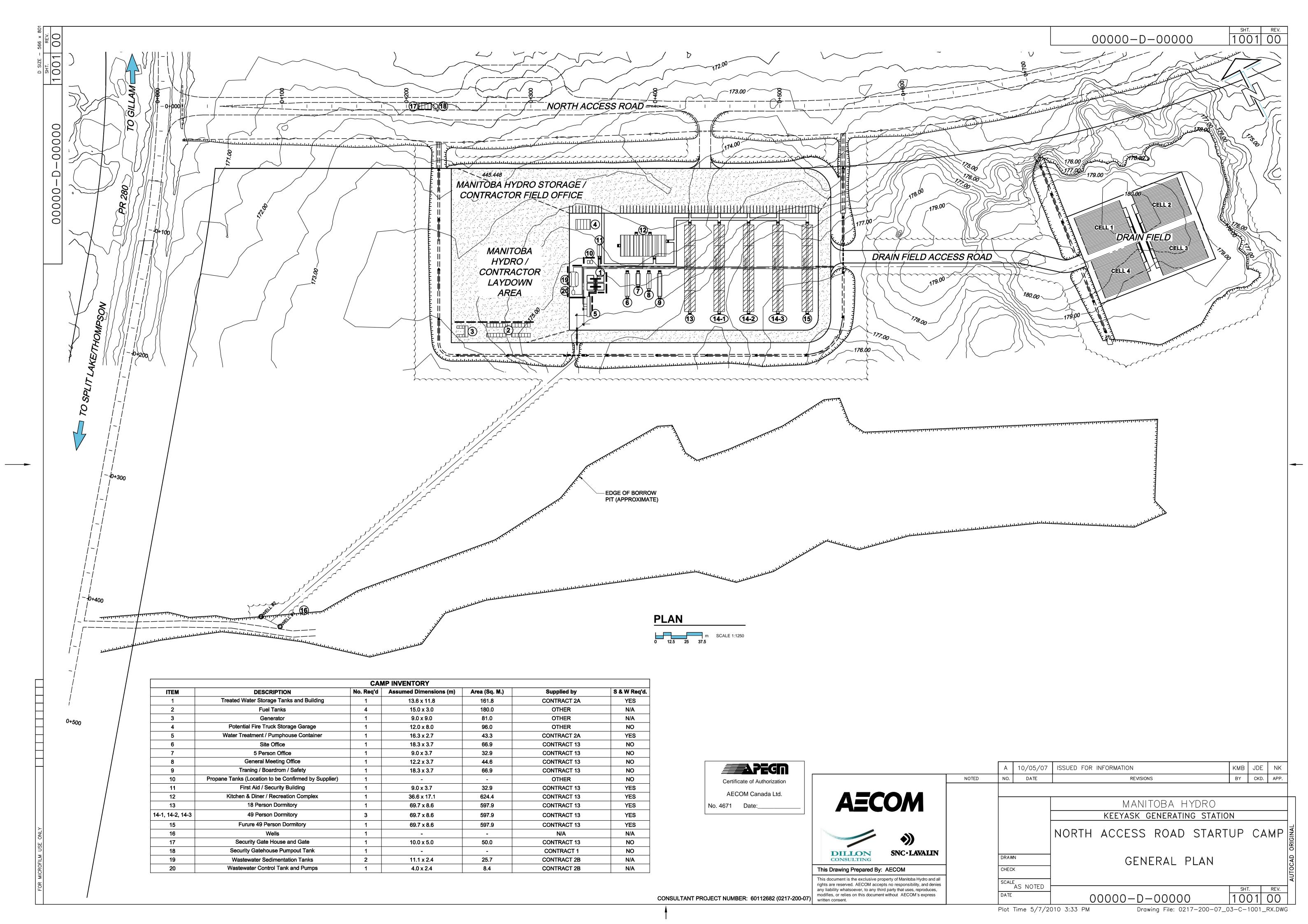
Figure 1 – Soil Texture Classification Triangle Drawings

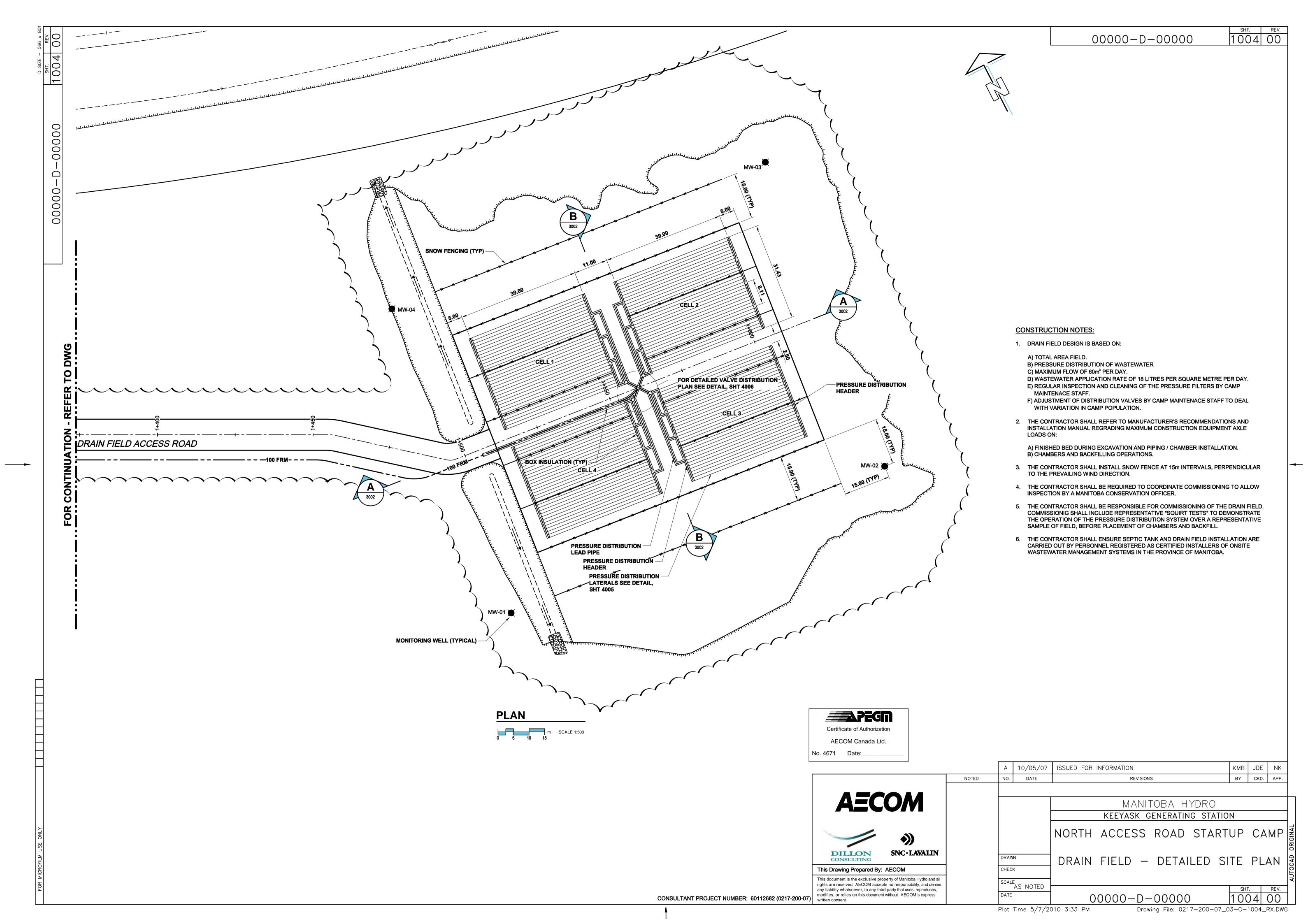
Figure 1

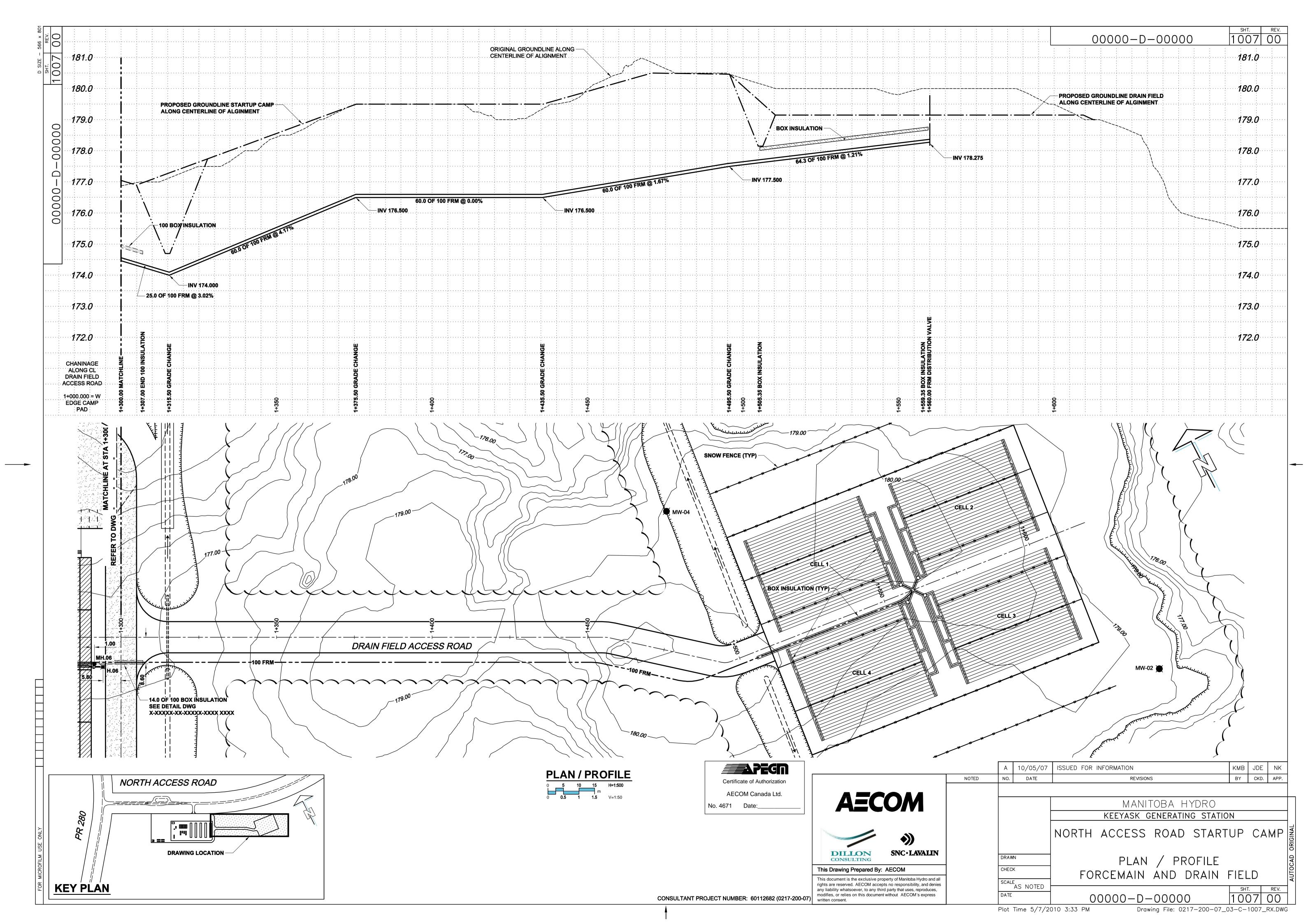
# SOIL TEXTURE CLASSIFICATION TRIANGLE

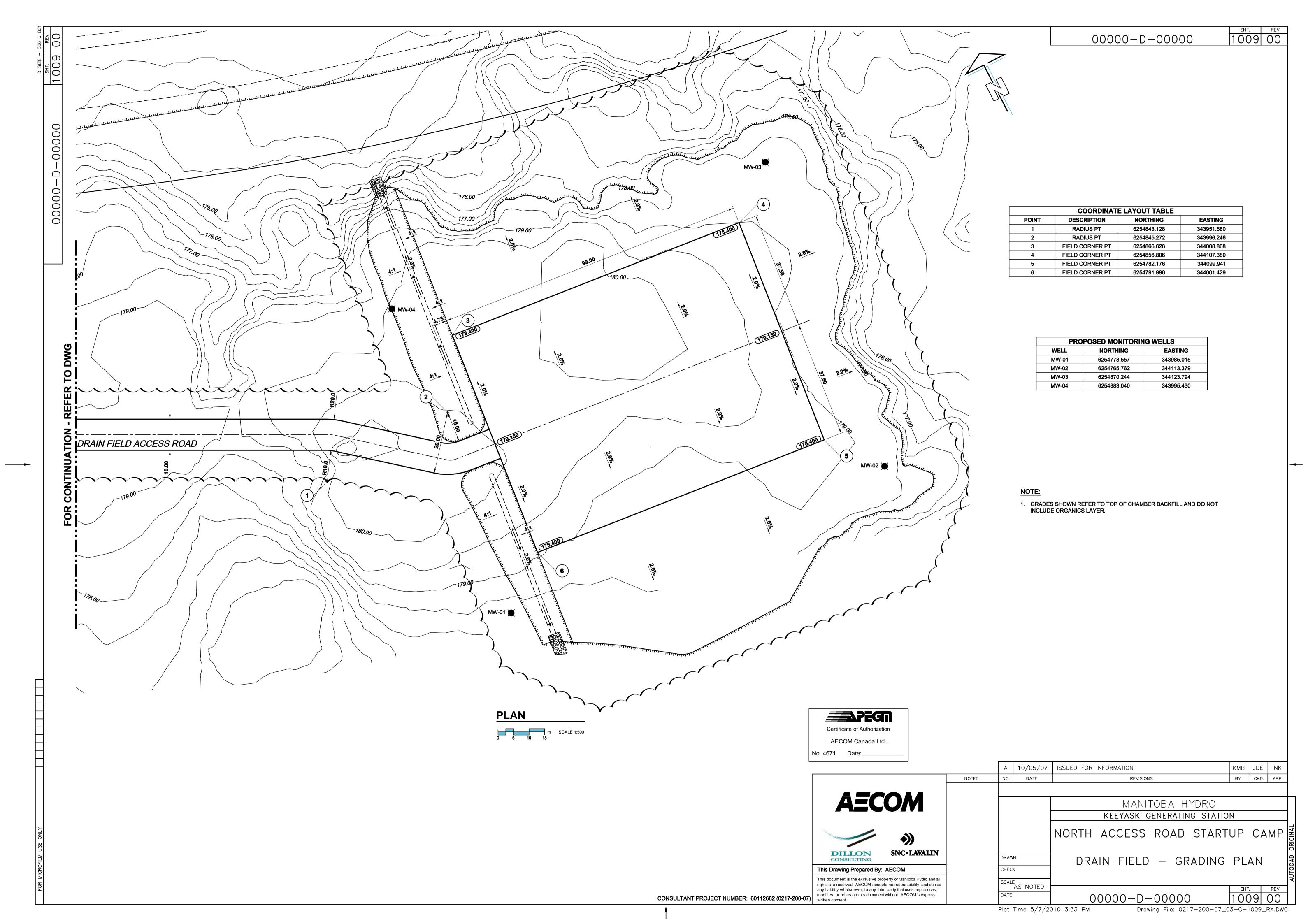


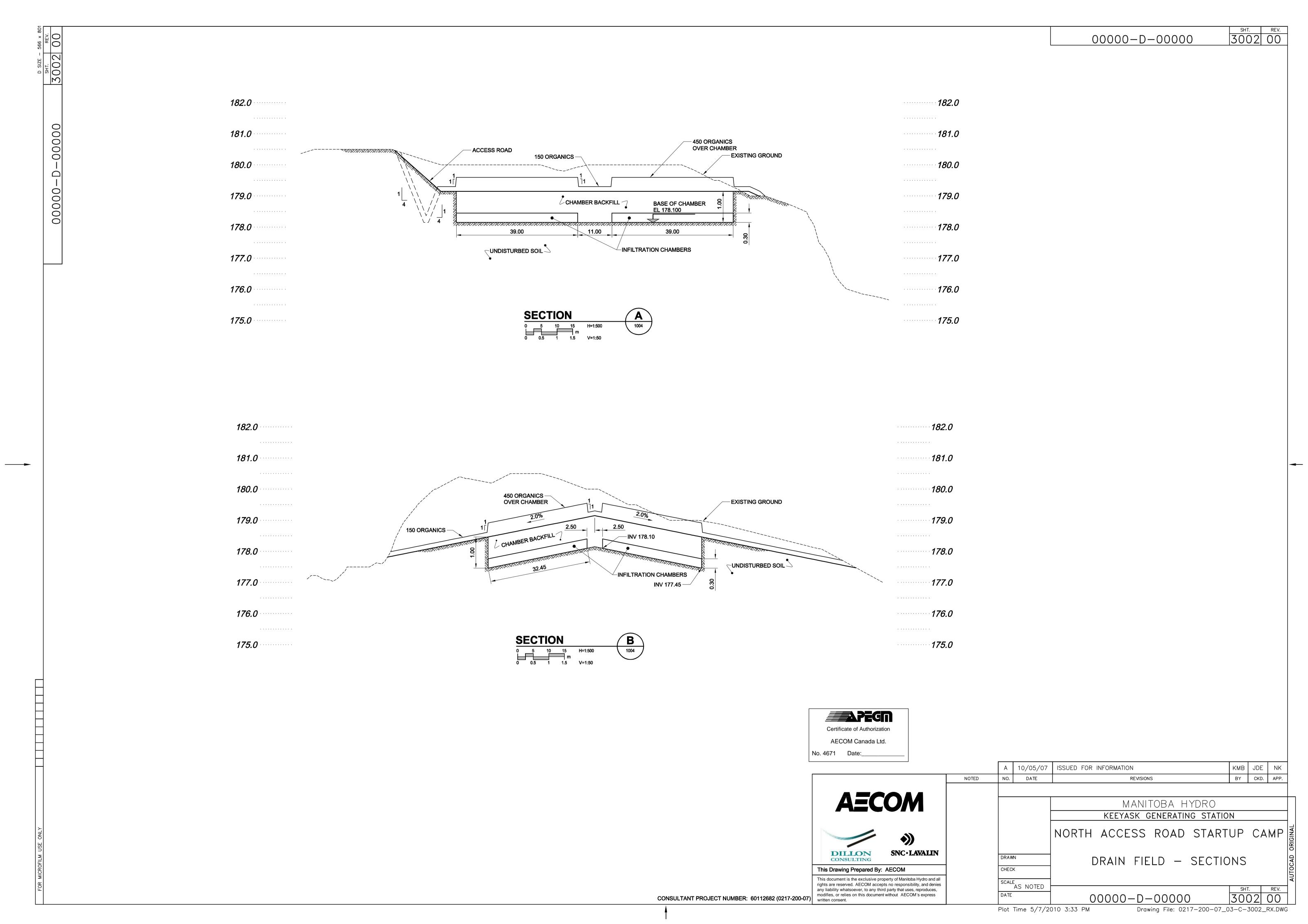
\*APPLICATION RATE-LITRES PER DAY PER SQUARE METRE

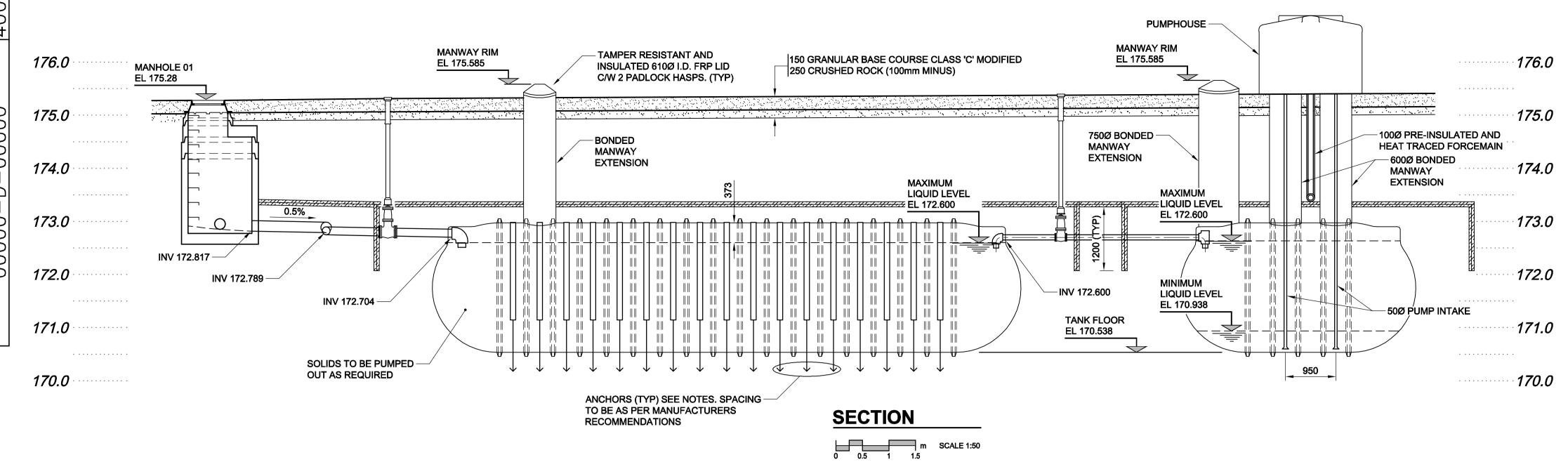


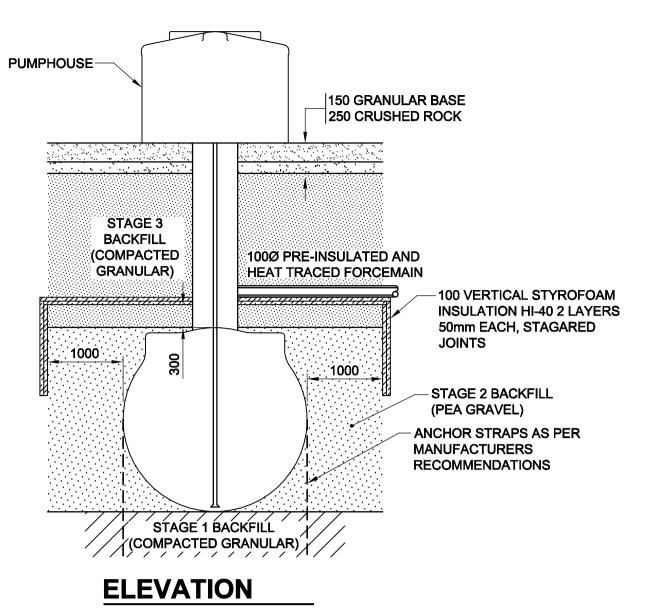


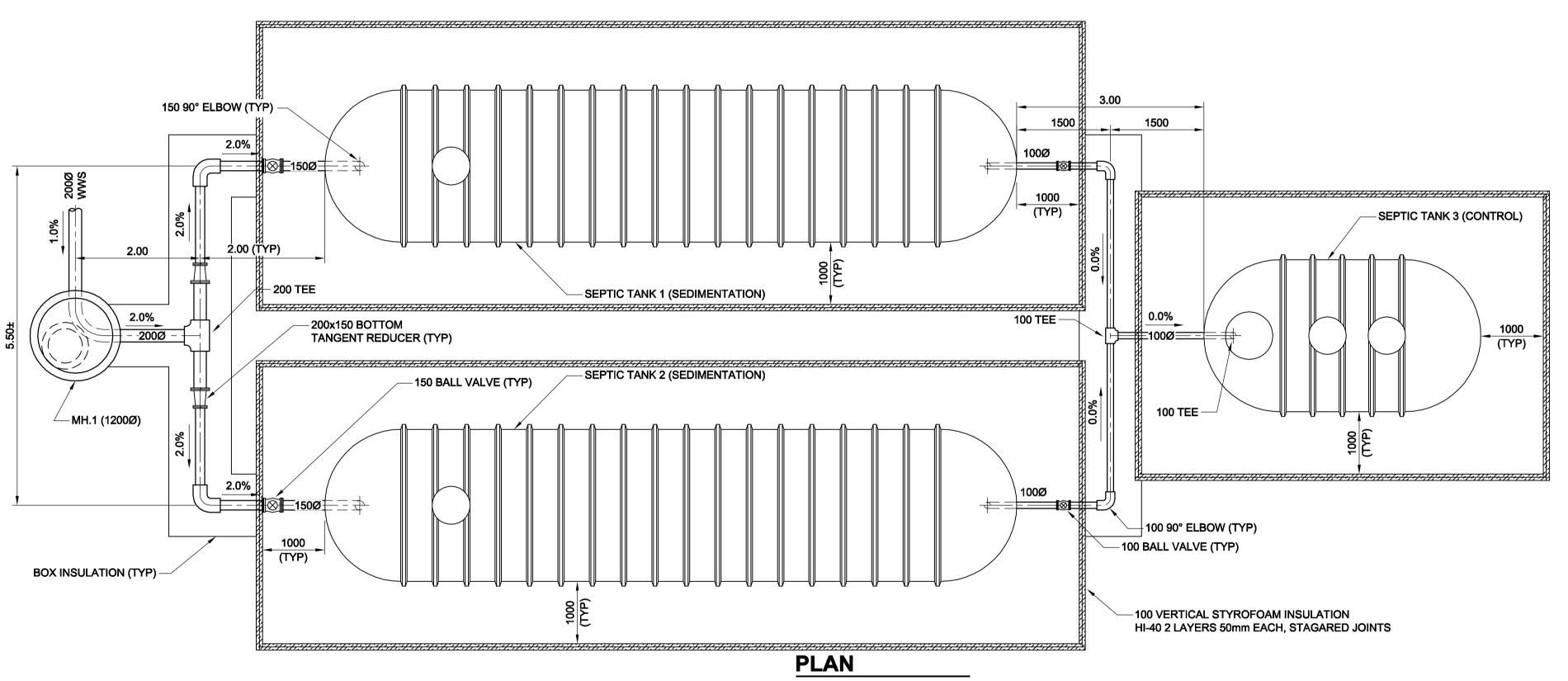












NOMINAL VOLUME

45,460 L

45, 460 L

13, 640 L

**TANK** 

2

3

### **DRAWING NOTES:**

- 1. EXCAVATE AS REQUIRED AND REMOVE FROM SITE MATERIALS NOT REQUIRED FOR BACKFILL OR GRADING.
- BACKFILL WITH MATERIALS INDICATED ON DRAWINGS AND SPECIFICATIONS.

0 0.5 1 1.5 SCALE 1:50

- COMPACTION (STANDARD PROCTOR DRY DENSITY %)
- 3.1. SUBGRADE BESIDE AND TOP OF TANK 90%
- 4. FILL IN 200mm LIFTS (LOOSE) AND COMPACT EACH LIFT TO REQUIRED DENSITY.

- 1. CONTRACTOR SHALL DESIGN, DETAIL, SUPPLY AND INSTALL ALL STAINLESS STEEL ANCHORS AND MISCELLANEOUS METAL HARDWARE OR PRECAST CONCRETE
- ANCHORS NECESSARY TO HOLD DOWN TANKS. 2. SUBMIT GENERAL LAYOUT SHOP DRAWINGS TO ENGINEER FOR APPROVAL PRIOR TO
- FABRICATION. THESE DRAWINGS SHALL INCLUDE: 2.1. ALL MISCELLANEOUS DETAILS.
- 2.2. SEAL OF PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA
- COVERING DESIGN OF LIFTING DEVICES AND NEOPRENE BEARINGS. 3. ALL PRECAST UNITS SHALL BEAR IDENTIFICATION MARK PLACED LEGIBLY ON UNIT AT TIME OF MANUFACTURE.

## PROCEDURES AND GENERAL NOTES

- EXCAVATE TO LEVEL BASE ON UNDISTURBED SUBGRADE CAPABLE OF 120 Kpa. BEARING PRESSURE.
- INSTALL ANCHORS AS PER MANUFACTURERS RECOMMENDATIONS.
- PLACE STAGE 1 BACKFILL AS SPECIFIED.
- PLACE TANK ON STAGE 1 BACKFILL AND SECURE STRAPS. BACKFILL AROUND TANK (STAGE 2) UNIFORMLY IN LAYERS AS SPECIFIED AROUND
- ALL SIDES TO A HEIGHT OF THE TANK.
- BACKFILL ABOVE TANK AS SHOWN (STAGE 3) USING SAME BACKFILL TECHNIQUE IN
- GRANULAR BACKFILL TO BE UNIFORMLY GRADED AGGREGATE WITH MAXIMUM
- AGGREGATE SIZE OF 63.5mm AND 7% MAXIMUM PASSING # 200 SIEVE. COMPACT TO 95% STANDARD PROCTOR DENSITY IN 150mm MAXIMUM LIFTS.

**HORIZON / POLYWEST** 

REFERENCE NUMBER

FGHON45460-8T2.14

FGHON45460-8T2.14

FGH0N13640-8P2.14

Certificate of Authorization AECOM Canada Ltd. No. 4671 Date:\_

> 10/05/07 ISSUED FOR INFORMATION KMB JDE NK BY CKD. APP. DATE REVISIONS **AECOM** MANITOBA HYDRO KEEYASK GENERATING STATION

NORTH ACCESS ROAD STARTUP CAMP DRAWN SEWER AND WATER - DETAILS 3 CHECK SCALE AS NOTED

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SNC+LAVALIN

CONSULTANT PROJECT NUMBER: 60112682 (0217-200-07) written consent.

DILLON

CONSULTING

SEPTIC TANK TABLE

TRICKLE

TRICKLE

PUMPOUT

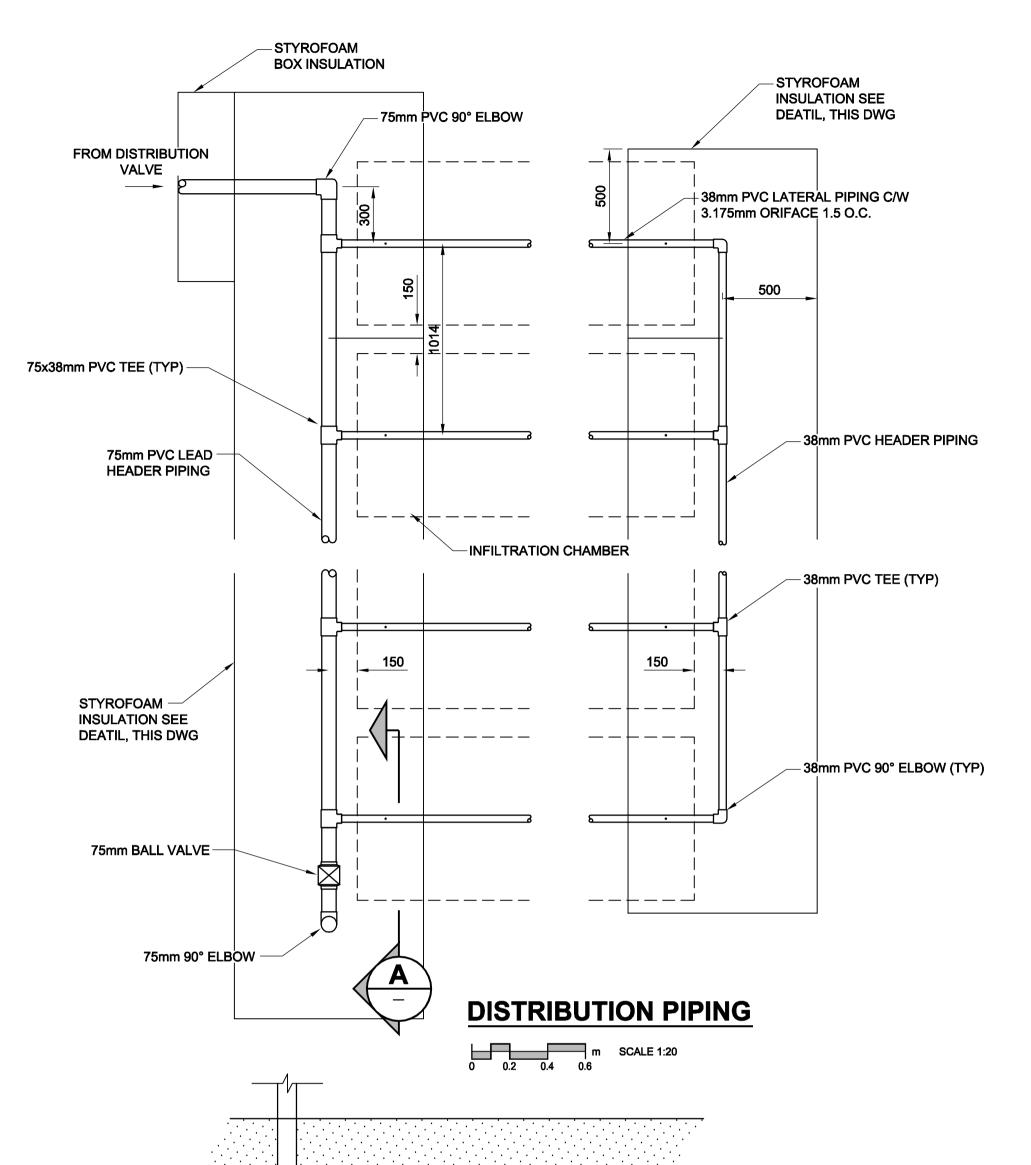
**OUTSIDE DIAMETER** 

2591

2591

2591





450 ORGANICS

- CLEANOUT ASSEMBLY

- VALVE BOX

100 STYROFOAM HI-40 INSULATION 50 EACH STAGARD JOINTS

- 38mm LATERAL PIPING

SUITABLE SITE

- 75mm 90° ELBOW

300

75mm PVC BALL VALVE

- 50x100x2450mm MARKER.

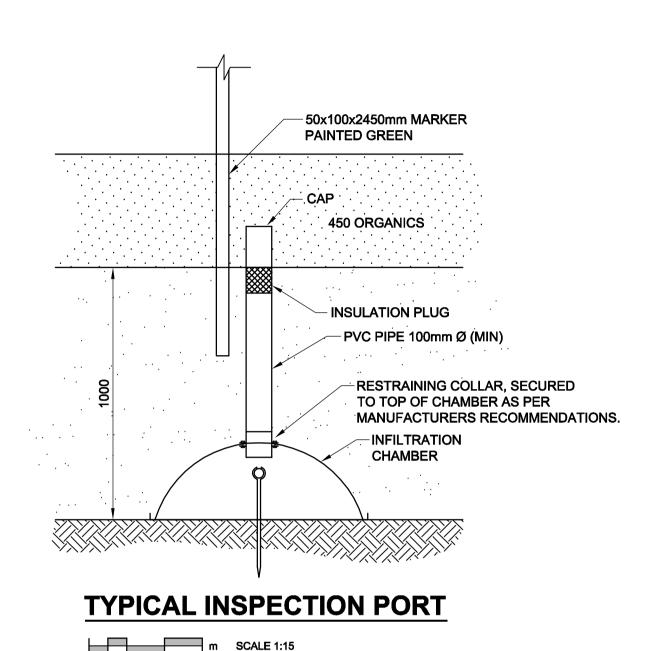
**DETAIL** 

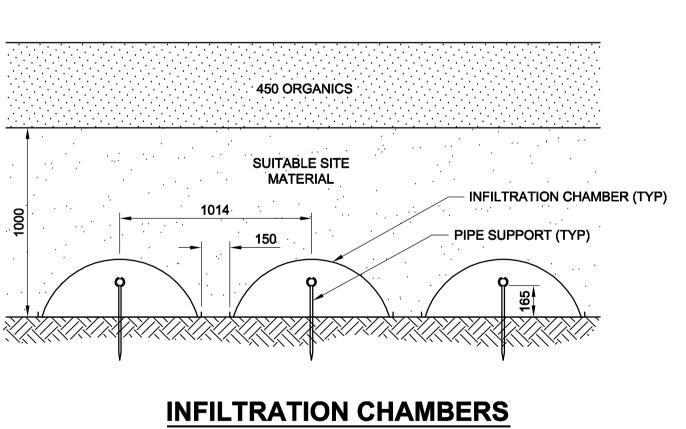
**HEADER CLEANOUT** 

4005

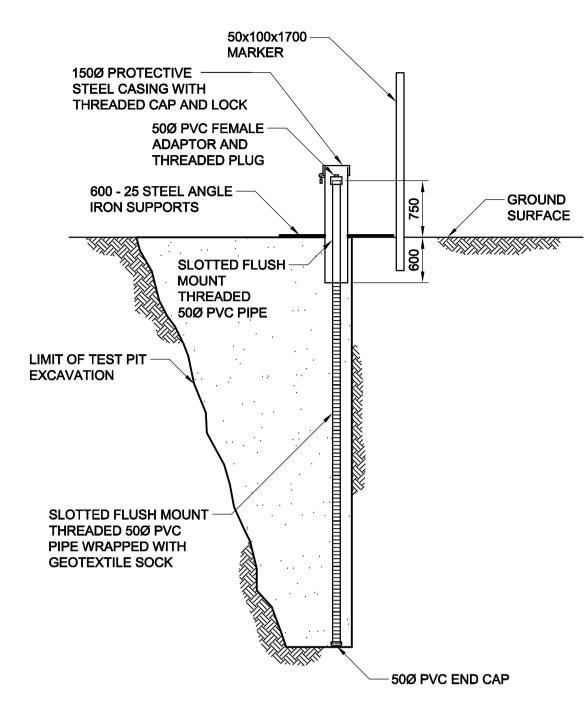
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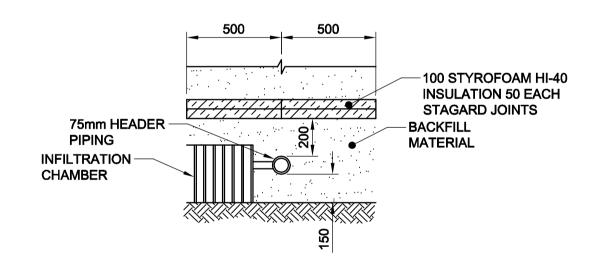




0 0.2 0.4 0.6 m SCALE 1:20



**MONITORING WELL** 



# HEADER INSULATION DETAIL m SCALE 1:20



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				KEEYASK GENERATING STATION				
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